

APPENDIX I

**EVALUATION OF HYDRAULIC PERFORMANCE
FOR SCA DESIGN**

GEOSYNTEC CONSULTANTS

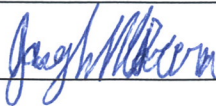
COMPUTATION COVER SHEET

Client: Honeywell Project: Onondaga Lake SCA Design Project/Proposal #: GJ4299 Task #: 18

TITLE OF COMPUTATIONS EVALUTION OF HYDRAULIC PERFORMANCE FOR SCA DESIGN

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APPROVAL NOTES:

REVISIONS (Number and initial all revisions)

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Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: Honeywell Project: Onondaga Lake SCA Final Design Project/ Proposal No.: GJ4299 Task No.: 18

EVALUATION OF HYDRAULIC PERFORMANCE FOR SCA DESIGN

INTRODUCTION

This package was prepared in support of the design of the Sediment Consolidation Area (SCA) for the Onondaga Lake Bottom Site, which will be constructed on Wastedbed 13 (WB-13). This package presents an analysis of hydraulic performance of the SCA, which will contain geotextile tubes (geo-tubes) filled with dredge material from Onondaga Lake. The analysis presented in this package has three main objectives:

1. Evaluate the infiltration rate through the final cover system. The infiltration rate through the cover system is needed to calculate the amount of liquid to be pumped through the base liner liquid management system (LMS) after closure of the SCA.
2. Evaluate the liquid head above the liner system. It should be noted that the maximum liquid head acting on the base liner system will be limited to 12 inches per New York State Department of Environmental Conservation (NYSDEC) regulations.
3. Determine the liquid head on the cover system of the SCA top and side slopes. The liquid head on the cover system of the SCA side slopes is required to analyze the veneer stability of the final cover system. More details regarding the veneer stability analysis are provided in the calculation package titled "Final Cover Veneer Stability Analyses for SCA Design" (Appendix L of the SCA Final Design).

In addition, calculations were performed to estimate the amount of consolidation water that could potentially be squeezed out of the foundation Solvay waste (SOLW) due to the SCA loading. The estimated amount of consolidation water was compared with the estimated reduction in infiltration of precipitation into WB-13 due to the installation of the SCA liner system. Calculations were also performed to compare the efficiencies of the proposed liner and NYSDEC Part 360 liner systems with regards to protection of groundwater quality.

METHODOLOGY

The calculations in this package were performed using the "Hydraulic Evaluation of Landfill Performance" (HELP) software, Version 3.07 developed by the U.S. Environmental Protection Agency. The HELP model is a quasi two-dimensional hydrologic model of water movement across, into, through, and out of landfills. The model accepts weather, soil, and design data and accounts for the effects of surface storage, snowmelt, runoff, infiltration,

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evapotranspiration, vegetative growth, lateral drainage, and leakage through liners [Schroeder, 1994]. The amount of consolidation water was estimated using methods that are consistent with the calculation package titled “Settlement Analyses for SCA” (Appendix H of the SCA Final Design).

INPUT PARAMETERS

The HELP software accepts parameters for layer type, hydraulic conductivity K for each layer, drainage path length, slope, moisture storage values, and climate. The SCA was modeled as shown in Figure 1, which provides a typical cross section. Specific properties of the eight different layers of materials are included in Table 1 and are discussed further below. It is noted that the HELP default parameters were used to select the total porosity, field capacity, and wilting point of each layer, however, the hydraulic conductivity was modified to better represent expected or potentially critical conditions.

SCA Geometry and Cover

The current design of the side slopes of the SCA is based on a 20-ft offset between geo-tubes. Each geo-tube layer is expected to be approximately 6-ft thick. This results in side slopes of 20 horizontal:6 vertical (i.e., 30% slopes) and a slope angle $\beta=16.7$ degrees. The drainage path length along the side slopes is 100 ft. The overall final cover slope on the top of the SCA is approximately 0.5% towards the low points near the sump areas. The longest possible drainage path is shown in Figure 2, which is approximately 1500 ft in length.

The current design of the SCA final cover consists of the following layers, from top to bottom, as shown in Figures 3A and 3B for the top and side slopes, respectively:

- Vegetative soil layer;
- Protective soil layer;
- Geocomposite drainage layer (SCA side slopes only);
- Geomembrane (GM) liner; and
- Leveling layer.

The proposed vegetative soil layer has a thickness of 6 in and was modeled as a sandy silt (ML) with a hydraulic conductivity of 1.0×10^{-4} cm/s. The protective soil layer has a thickness of 24 in and is assumed to be a sandy clay (SC) material with a hydraulic conductivity of 1.0×10^{-5} cm/s.

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The geocomposite drainage layer will only be included in the side slopes of the SCA to reduce the liquid head on the cover and improve the final cover veneer stability. For purposes of this analysis, a 200 mil geocomposite with a minimum measured hydraulic conductivity of 10 cm/s is recommended. After the use of reduction factors to account for creep (1.2), delayed intrusion (1.1), degradation (1.2), particulate clogging (1.2), chemical clogging (1.2), biological clogging (1.3) and a general safety factor of 2.5, the design hydraulic conductivity is considered to be 1.33 cm/s.

The cover GM was modeled using the HELP model parameters for a low density polyethylene (LDPE) liner, which is available in the HELP database. It is noted that changing the type of GM to other materials, such as ethylene-propylene diene monomer (EPDM) or high density polyethylene (HDPE), which are also available in the HELP database, did not affect the calculated values. It was assumed to contain one hole per acre and have good contact with the soil layer below, both of which can typically be achieved during construction. The holes in the GM were assumed to be 0.16 in², following the recommendations of Giroud and Bonaparte [1989].

A leveling layer was assumed to smooth post-settlement contours and maintain positive drainage. The decision on the material to be used in the leveling layer will be determined based on available material sources during operations. Therefore, a sensitivity analysis has been performed to model the impact of using either a SC material of the same type as the protective soil layer (i.e., hydraulic conductivity of 10⁻⁵ cm/s) or a granular material with hydraulic conductivity of 1.0 cm/s that is likely to be available if particle separation is performed on the dredged material. It is noted that the thickness of the leveling layer may vary, based on surface water requirements and the amount of separated material available for disposal. Based on sensitivity analyses, a thickness of 6 inches was selected for modeling.

Geo-tubes and Dredge Material

The geo-tubes above the gravel drainage layer were modeled as three separate layers, from bottom to top: (i) a layer for the dredge material; (ii) a small intermediate layer; and (iii) a geotextile layer for the top layer of geo-tubes. The 30-ft thick dredge material was modeled with properties similar to a low plasticity silt (ML). A hydraulic conductivity of $K_{\text{DREDGE}} = 1.0 \times 10^{-5}$ cm/s was used to represent the dredged material. Laboratory test results on the material to be dredged, which consists mostly of Solvay waste (SOLW) from the In-Lake Waste Deposit (ILWD) of Onondaga Lake, indicate that the hydraulic conductivity of the dredge material (K_{DREDGE}) may vary from approximately $K_{\text{DREDGE}} = 10^{-5}$ cm/s to $K_{\text{DREDGE}} = 10^{-6}$ cm/s. As discussed in subsequent sections, sensitivity analyses were performed to evaluate the effect of K_{DREDGE} on the infiltration rate and the head on the liner.

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The intermediate layer above the dredge material was modeled as a vertical percolation layer of 60-mil thickness of dredge material. This layer is necessary due to modeling restrictions within the HELP program, which does not allow the placement of a barrier soil layer underlying another barrier soil layer.

The geotextile layer represents the interface between the geo-tubes and dredge material. As the liquid flows through the cover system to reach the top geo-tube/dredge material interface, it will experience preferential flow patterns. As shown in Figure 4, the liquid can pass through the geotextile material itself or through the open area filled with dredge material. Therefore, this layer was modeled using a combined equivalent hydraulic conductivity K_{EQ} , calculated using Equation 1 below, as discussed by Das [2005].

$$K_{EQ} = K_{DREDGE} \times A_{DREDGE} + K_{GT} \times A_{GT} \quad (\text{Eq.1})$$

where:

- K_{EQ} = Equivalent hydraulic conductivity
- K_{DREDGE} = Hydraulic conductivity of dredge material
- A_{DREDGE} = Cross-sectional area of dredge material, measured as percent open area of the geotextile
- K_{GT} = Hydraulic conductivity of woven geotextile
- A_{GT} = Cross-sectional area covered by woven geotextile material and calculated as $100\% - A_{DREDGE}$

The percent open area was assumed to be 4% based on the standard typical minimum value for a woven geotextile [TC Mirafi, 2000]. A sensitivity analysis on the equivalent hydraulic conductivity resulting from different values of K_{GT} was performed and is shown in Figure 5. As described on Figure 5, a value of $K_{EQ} = 4.0 \times 10^{-7}$ cm/s was selected based on this analysis.

Liner System

The SCA liner system consists of the following layers from top to bottom:

- Gravel drainage layer;
- Nonwoven geotextile cushion;
- Geomembrane (GM) liner; and
- Low permeability soil layer.

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The proposed gravel drainage layer has an average thickness of 2 ft and a minimum thickness of 1 ft. It was modeled conservatively as a 1-ft thick layer. This drainage layer will be sloped towards the sumps to allow gravity drainage. Based on the predicted grading of the underlying low permeability soil layer 30 years after closure, the gravel drainage layer was modeled with a slope of 1% and a drainage path length of 1500 ft, as shown in Figure 6. A sensitivity analysis using a slope of 0.5% for the gravel drainage layer was also performed. The assumed hydraulic conductivity of the gravel is 10 cm/s based on available material sources. Sensitivity analyses were performed to evaluate the effect of K_{GRAVEL} ranging from 5 cm/s to 15 cm/s, based on a range in values for other potentially available gravel types.

A nonwoven (NW) geotextile (GT) cushion will be placed between the gravel drainage layer and GM for protection. This NW GT layer was not included in the HELP model.

The GM was modeled using the HELP model parameters for a low density polyethylene (LDPE) liner, which is available in the HELP database. It is noted that changing the type of GM to other materials, such as ethylene-propylene diene monomer (EPDM) or high density polyethylene (HDPE), which are also available in the HELP database, did not affect the calculated values. The GM was assumed to contain one hole per acre and have good contact with the soil layer below, both of which can typically be achieved during construction. The holes in the GM were assumed to be 0.16 in², following the recommendations of Giroud and Bonaparte [1989].

The low permeability soil layer has a proposed minimum thickness of 1 ft. The top 6 inches of this layer is required to have a hydraulic conductivity of $K=10^{-6}$ cm/s or lower, per agreement with NYSDEC. The bottom part of this layer was modeled with a hydraulic conductivity of $K=10^{-5}$ cm/s.

Climate Data

The data for precipitation, temperature, humidity, and solar radiation were modeled using the HELP software synthetic data generation function for Syracuse, New York for a 100-year modeling period. This generation uses recorded mean monthly data for Syracuse, shown in Table 2, to stochastically generate daily data with approximately the same statistical characteristics as the historic data. The precipitation data was manually adjusted to account for the design storm event of a 25-year, 24-hour rainfall. The rainfall corresponding to the design storm event was selected to be 4.4 inches based on recommendations from the Natural Resources Conservation Service [1986], as shown in Figure 7. The evaporative zone depth, which is the maximum depth from which water may be removed by evapotranspiration, was assumed to be 30

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inches, corresponding to the total thickness of the topsoil and the protective soil layer in the final cover system. For this analysis, it is assumed that a good stand of grass will be established on the final cover system; therefore, the leaf area index (i.e., the dimensionless ratio of leaf area of active vegetation to the nominal surface area) of 3.5 was selected. If willows are planted above the GM, less infiltration will result; therefore, that case is not analyzed herein. Values of the climate parameters are shown in Table 3.

CASES ANALYZED

Table 4 provides a summary of the cases evaluated, which includes a Base Case (top and side slopes) and a sensitivity analysis on the parameters K_{DREDGE} , K_{GRAVEL} , and $K_{LEVELING}$. Case 1, as compared to the Base Case (top), was used to evaluate the effect of uncertainty in the hydraulic conductivity of the dredge material. Cases 2A and 2B, as compared to the Base Case (top), were used to evaluate the effect of the gravel drainage layer hydraulic conductivity on liquid production and the liquid head on the geomembrane liner. Case 3, as compared to the Base Case (top), was used to evaluate the effect of a clayey soil (i.e., SC soil, $K_{LEVELING}=1.0 \times 10^{-5}$ cm/s) in the leveling layer as compared to granular soil (i.e., $K_{LEVELING}=1.0$ cm/s) in the leveling layer.

For analysis of the SCA side slopes, the Base Case parameters were used with a geocomposite added to the final cover, as discussed previously. This case is referred to as Base (side slopes) in Table 4.

Finally, HELP modeling was also performed to demonstrate the efficiency of the SCA with the proposed composite liner system as compared to the efficiency of the SCA with a prescribed Part 360 liner. The Part 360 prescribed liner system used for comparison is summarized in Table 5.

RESULTS OF ANALYSIS

The results of the HELP Model analyses are summarized in Table 6. HELP Output files for the Base Case on the SCA top (run I_1A) and SCA side slopes (run III_1A) are included in Attachment 1.

The effect of uncertainty in the hydraulic conductivity of the dredge material was studied by comparing the results of Case 1 with the Base Case (top). Increasing the hydraulic conductivity of the dredge material did not significantly affect calculated peak daily or average annual values

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of liquid head on the cover and liner system or liquid production. Therefore, it is recommended that a value of $K_{DREDGE} = 10^{-5}$ cm/s be used for design purposes.

When compared to the Base Case on the top ($K_{GRAVEL} = 10$ cm/s), changing the gravel hydraulic conductivity (i.e., Cases 2A and 2B) did not significantly affect calculated peak daily or average annual values of liquid head on the cover and liner system or liquid production. Therefore, it is recommended that a value of $K_{GRAVEL} = 10$ cm/s be used for design purposes.

Changing the material used in the leveling layer (i.e., Base Case [top] compared to Case 3) did not affect the calculated peak daily or average annual values of liquid head on the cover and liner system or liquid production. Therefore, based on the results of the HELP analysis, use of either the SC soil or a granular material (or any other soil with K values in between 1.0×10^{-5} cm/s and 1 cm/s) is considered to be acceptable from a hydraulic performance standpoint.

The results of the consolidation water and infiltration reduction calculations are presented in Table 7. These calculations were performed on an annual average basis during the SCA operational period using reasonable assumptions regarding liner construction and geo-tube filling phasing. Key assumptions are presented as footnotes in Table 7. During the four years of operation, total amounts of consolidation water due to filling of the SCA and infiltration reduction due to installation of the liner were calculated to be 189 and 162 million gallons, respectively. Considering the complicated nature of nonlinear field processes such as consolidation and infiltration and the use of idealized simple models to perform the estimates, the total amount of consolidation water and reduction in infiltration are considered to be comparable over a four year operational period. The current capacity of the existing liquid collection system and its ability to handle a potential increase in flow will be further evaluated as part of the closure of WB-9 through WB-15.

The equivalency calculations summarized in Table 8 indicate that the SCA with both the proposed liner and the NYSDEC Part 360 liner have liner efficiencies of greater than 99.99%. Therefore, it can be demonstrated that the proposed system will effectively protect groundwater quality.

SUMMARY AND CONCLUSIONS

This package presents the analyses of the hydraulic performance of the proposed SCA using the HELP model. The evaluation presented in this package has three main objectives: (i) evaluate the infiltration rate through the proposed final cover system; (ii) evaluate the liquid head above the liner system; and (iii) evaluate the liquid head above the GM in the cover system on the top and side slopes of the SCA. Analysis of the SCA top and side slopes indicates that the

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calculated liquid head on the liner satisfies the NYSDEC requirement of 1 ft or less on the liner system. The calculated annual average liquid rate was approximately 0.4 gallons per minute. The maximum and average calculated liquid heads in the cover system on the SCA side slopes during the peak day were 0.030 inches (0.003 ft) and 0.020 inches (0.002 ft), respectively.

In addition, the calculations performed to estimate the amount of consolidation water indicate that the amount of water squeezed out over the four year operational period is the same order of magnitude as the reduction in infiltration that will occur due to liner system installation. The capacity of the existing liquid collection system is currently being evaluated as part of the WB-9 through WB-15 closure.

The equivalency calculations indicate that the efficiency of the proposed SCA with its composite liner system is comparable to the efficiency of the SCA with a Part 360 liner. Therefore, it can be demonstrated that the proposed SCA liner will effectively protect groundwater quality at the site.

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Tables

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Table 1: Material Properties of Layers within SCA System

| Layer | Description ^[1] | Layer Type ^[2] | HELP Default ^[3] | HELP USCS Name | Thickness (in) | K (cm/s) |
|-------|-----------------------------------|---------------------------|-----------------------------|----------------|------------------|------------------------|
| 1 | Topsoil | 1 | 8 | ML | 6 | 1.0E-04 |
| 2 | Protective Layer | 2 | 13 | SC | 24 | 1.0E-05 |
| 3 | Geomembrane | 4 | 36 | LDPE | 0.06 | 4.0E-13 ^[4] |
| 4 | Leveling Layer | 1 | 21 / 13 | Gravel / SC | 6 ^[5] | 1.0 / 1.0E-05 |
| 5 | Geo-tube with Dredge Filter Cake | 3 | 20 | Drainage Net | 0.06 | 4.0E-07 ^[6] |
| 6 | Dredge material ^[7] | 1 | 22 | ML | 0.06 | 1.0E-05 ^[8] |
| 7 | Dredge material | 3 | 22 | ML | 360 | 1.0E-05 ^[8] |
| 8 | Gravel Drainage | 2 | 21 | Gravel | 12 | 10 ^[9] |
| 9 | Geomembrane | 4 | 36 | LDPE | 0.06 | 4.0E-13 ^[4] |
| 10 | Low Permeability Soil (Top 6") | 3 | 24 | SC | 6 | 1.0E-06 |
| 11 | Low Permeability Soil (Bottom 6") | 1 | 13 | SC | 6 | 1.0E-05 |

Notes:

1. This is a general description of each layer.
2. The following layer types are available in the HELP model: 1=Vertical Percolation, 2=Lateral Drainage, 3=Barrier Soil Liner, 4=Geomembrane Liner (GM).
3. This is the HELP default soil texture number. It is noted that the hydraulic conductivity of each layer may be changed from the HELP default to better represent expected or potentially critical conditions. All input parameters can be found in the HELP output files provided in Attachment 1.
4. This layer is modeled as LDPE GM, using typical values from the HELP database. Selection of a different GM type will not affect the results significantly.
5. The leveling layer thickness may vary based on surface water requirements and the amount of separated material available for disposal. Based on sensitivity analyses discussed in the package, a thickness of 6 inches was selected for modeling.
6. This was calculated using the percent Open Area, as discussed in the package.
7. This layer is identical to the 360 inch (30 ft) layer of dredge material, but is modeled as a vertical percolation layer. This is necessary due to modeling restrictions within the HELP program.
8. The value shown here is the base parameter, used in cases labeled "A". This value was changed to $K=1.0 \times 10^{-6}$ cm/s in cases labeled "B" to evaluate sensitivity.
9. The assumed hydraulic conductivity of the gravel is 10 cm/s, based on potentially available material.
10. It is noted that layers 8 through 11 compose the proposed liner system.

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11. It is noted that for the side slope case (S_III_1A), a 200 mil geocomposite drainage layer was added between layers 2 and 3. This layer was modeled as the HELP default soil texture 20 (i.e., drainage net) and a design hydraulic conductivity of 1.33 cm/s, which corresponds to a measured laboratory hydraulic conductivity of 10 cm/s.

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Table 2: Normal Mean Precipitation and Temperature Data for Syracuse, NY

| Month | Precipitation (in) | Temperature (°F) |
|-----------|--------------------|------------------|
| January | 2.61 | 22.80 |
| February | 2.65 | 24.00 |
| March | 3.11 | 33.30 |
| April | 3.34 | 46.10 |
| May | 3.16 | 57.00 |
| June | 3.63 | 66.30 |
| July | 3.76 | 70.90 |
| August | 3.77 | 69.30 |
| September | 3.29 | 62.10 |
| October | 3.14 | 51.30 |
| November | 3.45 | 40.60 |
| December | 3.20 | 28.30 |

Note:

These are the default normal mean monthly values of precipitation and temperature for Syracuse, NY, available in the HELP software.

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Table 3: Climate Parameters for Syracuse, NY and Other Parameters for the SCA Final Cover
(HELP Default Values Unless Stated Otherwise)

| Parameter | Value | Units |
|--|--------------------|-----------------------|
| Fraction of Area allowing Runoff | 100% | percent of total area |
| Evaporative Zone Depth | 30 | Inches |
| Latitude | 43.07 | Degrees |
| Maximum Leaf Area Index | 3.5 ^[3] | -- |
| Start of Growing Season (date) | 124 ^[4] | -- |
| End of Growing Season (date) | 284 ^[4] | -- |
| Planar Area ^[1] | 1 | Acre |
| Average Annual Wind Speed | 9.7 | miles/hr |
| Average 1st Quarter Relative Humidity | 72% | -- |
| Average 2nd Quarter Relative Humidity | 68% | -- |
| Average 3rd Quarter Relative Humidity | 75% | -- |
| Average 4th Quarter Relative Humidity | 76% | -- |
| Peak Daily Rainfall, 25-year, 24-hour storm event ^[2] | 4.40 | Inches |

Notes:

1. The area was modeled as one acre to produce values on a per-acre basis. This was multiplied by the total number of acres to calculate total flow rates over the entire area of the SCA. The total area of the SCA was assumed to be approximately 70 acres.
2. Value from National Resources Conservation Service data shown in Figure 7.
3. This value corresponds to a good stand of grass on top of the final cover.
4. These dates correspond to May 4 through October 11.

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Table 4: Cases Evaluated

| Case | Run ID ^[1] | Variable | Description of Variable | K _{DREDGE} (cm/s) | K _{GRAVEL} (cm/s) | K _{LEVELING} (cm/s) |
|--|-----------------------|-----------------------|---|-------------------------------|-------------------------------|---------------------------------|
| Base Top ^[2] | I_1A | N/A | Base case using parameters from Table 1 | 1.0E-05 | 10 | 1 |
| 1 | I_1B | K _{DREDGE} | Hydraulic conductivity of dredge material | 1.0E-06 | 10 | 1 |
| 2A | I_2A | K _{GRAVEL} | Hydraulic conductivity of gravel drainage layer | 1.0E-05 | 5 | 1 |
| 2B | I_2B | K _{GRAVEL} | Hydraulic conductivity of gravel drainage layer | 1.0E-05 | 15 | 1 |
| 3 | II_1A | K _{LEVELING} | Material to be used for leveling layer (SC or granular) | 1.0E-05 | 10 | 1.0E-05 |
| Base Side Slopes ^[3] | S_III_1A | N/A | Base case using parameters from Table 1 | 1.0E-05 | 10 | 1 |

Note:

1. The Run ID corresponds to the case listed in Table 6.
2. This is the base case for comparison. The parameters used are the base parameters shown in Table 1, and the leveling layer is considered to be granular material.
3. This is the base case for the side slopes. The parameters used are the base parameters shown in Table 1, with the modifications described in Note 11 of Table 1.

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Table 5: Part 360 Prescribed Liner System

| Layer | Description ^[1] | Layer Type ^[2] | HELP Default ^[3] | HELP USCS Name | Thickness (in) | K (cm/s) |
|------------------|----------------------------|---------------------------|-----------------------------|----------------|----------------|------------------------|
| 8 ^[4] | Granular Drainage | 2 | 1 | SP | 24 | 0.01 ^[5] |
| 9 | Geomembrane | 4 | 36 | LDPE | 0.06 | 4.0E-13 ^[6] |
| 10 | Compacted Clay | 3 | 29 | CH | 6 | 1.0E-07 ^[5] |
| 11 | Structural Fill | 1 | 10 | SC | 12 | 1.0E-04 ^[7] |
| 12 | Granular Drainage | 2 | 1 | SP | 12 | 0.01 ^[5] |
| 13 | Geomembrane | 4 | 36 | LDPE | 0.06 | 4.0E-13 ^[6] |
| 14 | Compacted Clay | 3 | 29 | CH | 24 | 1.0E-07 ^[5] |

Notes:

1. This is a general description of each layer.
2. The following layer types are available in the HELP model: 1=Vertical Percolation, 2=Lateral Drainage, 3=Barrier Soil Liner, 4=Geomembrane Liner (GM).
3. This is the HELP default soil texture number. It is noted that the hydraulic conductivity of each layer may be changed from the HELP default to better represent expected or potentially critical conditions.
4. Layers 1 through 7 represent the final cover system and geo-tubes. Therefore, they are modeled identically as in the base case. Details of these layers are shown on Table 1. Layers 8 through 14 compose the Part 360 prescribed liner system, as compared to Layers 8 through 11 in Table 1 that represent the proposed liner system.
5. Required values per NYSDEC Part 360 regulations.
6. This layer is modeled as LDPE GM, using typical values from the HELP database. It is noted that the regulations do not specify a GM material. Selection of a different GM type will not affect the results significantly.
7. This hydraulic conductivity value is assumed based on the type of material.

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Client: **Honeywell** Project: **Onondaga Lake SCA Final Design** Project/ Proposal No.: **GJ4299** Task No.: **18**

Table 6: Results of HELP Modeling for the SCA

| Case | Run ID | Curve Number | K _{DREDGE} (cm/s) | K _{GRAVEL} (cm/s) | K _{LEVELING} (cm/s) | Peak Daily Values | | | | | Average Annual Values | | | | |
|------------------|----------|--------------|-------------------------------|-------------------------------|---------------------------------|--------------------------------|--------------------------------|--------------------------------|---------------------------------|---------------------|--------------------------------|------------------------------------|------------------------|--------------------|---------------------|
| | | | | | | H _{AVG} (in) cover | H _{MAX} (in) liner | H _{AVG} (in) liner | Liquid (ft ³ /ad) | Liquid (gal/min) | H _{AVG} (in) liner | Liquid (ft ³ /ac-yr) | Liquid (gal/ac-day) | Liquid (gal/ac) | Liquid (gal/min) |
| Base Top | I_1A | 68.2 | 1.0E-05 | 10 | 1 | 30.0 | 0.0 | 0.0 | 2.6 | 0.9 | 0.0 | 366.0 | 7.5 | 525.0 | 0.4 |
| 1 | I_1B | 68.2 | 1.0E-06 | 10 | 1 | 30.0 | 0.0 | 0.0 | 2.6 | 0.9 | 0.0 | 366.0 | 7.5 | 525.0 | 0.4 |
| 2A | I_2A | 68.2 | 1.0E-05 | 5 | 1 | 30.0 | 0.0 | 0.0 | 2.6 | 0.9 | 0.0 | 366.0 | 7.5 | 525.0 | 0.4 |
| 2B | I_2B | 68.2 | 1.0E-05 | 15 | 1 | 30.0 | 0.0 | 0.0 | 2.6 | 0.9 | 0.0 | 366.0 | 7.5 | 525.0 | 0.4 |
| 3 | II_1A | 68.2 | 1.0E-05 | 10 | 1.0E-05 | 30.0 | 0.0 | 0.0 | 2.6 | 0.9 | 0.0 | 366.0 | 7.5 | 525.0 | 0.4 |
| Base Side Slopes | S_III_1A | 76.1 | 1.0E-05 | 10 | 1 | 0.02 | 0.0 | 0.0 | 13.1 | 4.8 | 0.0 | 403.5 | 8.3 | 578.8 | 0.4 |

Notes:

1. The curve number is calculated by HELP based on model inputs.
1. Selected maximum precipitation for a single day is 4.4 inches, based on the National Resources Conservation Service [1986] recommendations for a 25 year, 24 hour storm event.
2. SCA is assumed to have a total acreage of 70 acres.
3. This table uses a drainage path length of 1500 ft and slope of approximately 0.5% for the final cover top area and a drainage path length of 1500 ft and slope of 1% for the gravel drainage layer.
4. The HELP Output file for the base case, I_1A, is included in Attachment 1.
5. Case S_III_1A represents the SCA side slopes and is modeled with a final cover drainage path length of 100 ft and slope of 30%. It is noted that the gravel drainage layer is modeled with a drainage path length of 1500 ft and slope of 1%. The HELP Output file for this case is included in Attachment 1.

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Table 7: Results of Consolidation Water and Reduction in Infiltration Rate Calculations

| Year | SCA Footprint at End of Each Phase (ac) ^[1] | New SCA Footprint in Each Year (ac) | Total Lined SCA Area at End of Each Phase (ac) ^[2] | Additional Lined Area (ac) ^[3] | Infiltration Cut-off (million gallons) | | Consolidation Squeezed Water (million gallons) | | | | |
|--|--|-------------------------------------|---|---|--|--|---|--|---|--|---|
| | | | | | Assuming 40" Infiltration Each Year ^[4] | Assuming 0.5 x 40" Infiltration Each Year ^[5] | Estimated Average Geo-tube Height (ft) ^[6] | Estimated Average Total Thickness of Gravel Drainage Layer and Liner (ft) ^[7] | Estimated Total Equivalent Soil Thickness (ft) ^[8] | Estimated Average Settlement (ft) ^[9] | Estimated Consolidation Squeezed Water in Each Year (million gallons) ^[10] |
| 0 | -- | -- | 15 | 10 | 14 | 7 | -- | -- | -- | -- | -- |
| 1 | 30 | 30 | 40 | 10 | 54 | 27 | 18 | 5 | 18.1 | 5.4 | 53 |
| 2 | 50 | 20 | 61 | 10 | 77 | 39 | 22 | 5 | 20.7 | 6.2 | 48 |
| 3 | 72 | 22 | 72 | 10 | 89 | 45 | 23 | 5 | 21.3 | 6.4 | 49 |
| 4 | 72 | 0 | 72 | 10 | 89 | 45 | 30 | 5 | 26.8 | 8.0 | 38 |
| Total Estimated Infiltration Cut-off in 4+ years (million gallons) | | | | | 323 | 162 | Total Estimated Consolidation Squeezed Water in 4 years (million gallons) | | | | 189 |

Notes:

1. The SCA footprint is measured from the outside toe of the perimeter berm.
2. Assuming half of the new SCA footprint will be lined in advance during the previous year.
3. Additional lined area for basins, and water treatment and pre-processing facilities.
4. The infiltration cut-off volume equals 40 inches of annual average rainfall multiplied by the total lined area at the end of each year. In year 0, only half a year for the infiltration cut-off was assumed.
5. Assuming 50% for evapotranspiration based on the analysis presented in the HELP calculations.
6. To estimate the average geo-tube height for calculating loads for settlements, a 25% decrease of the available footprint was considered to account for the SCA perimeter berm, the 10-ft gap between geo-tubes and the berm, and the 20-ft offset distance for geo-tube stacking. This assumption is considered reasonable based on the calculation of an average geo-tube height of 30 ft for year 4.
7. These thicknesses are based on the calculations presented in the package titled "Volume Calculations for SCA Design".
8. Assuming a unit weight of 120 pcf for liner and gravel and 86 pcf for dredge material. The total thickness was converted to an equivalent thickness of soil with a unit weight of 120 pcf.
9. Using a rule of thumb: 10 ft soil causes approximately 3 ft of settlement in SOLW (based on test plot results). The total calculated average settlement of 8 feet in the 4th year is consistent with the results of detailed settlement calculations presented in the calculation package titled "Settlement Analyses for SCA" in the Final SCA Design.
10. The Solvay waste above the groundwater table was conservatively assumed to be fully saturated in the calculation. Primary consolidation time was assumed to be less than one year, thus resulting in all calculated consolidation water for a given year being squeezed out within that year.

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Table 8: Results of Liner Equivalency Calculations

| Run ID | Liner Type | Peak Daily Values | | | Average Annual Values | | |
|-----------------|---------------------------------------|------------------------------|------------------------|-------------------|--------------------------|--------------------|-------------------|
| | | Precipitation (in/ac-day) | Leakage (in/ac-day) | Efficiency (%) | Precipitation (in/ac) | Leakage (in/ac) | Efficiency (%) |
| I_1A | Proposed | 4.40 | 0.000314 | 99.9929% | 39.20 | 0.00340 | 99.9913% |
| 360_Soil | Part 360 - Double-liner with soils | 4.40 | 0.000000 | 100.0000% | 39.20 | 0.00001 | 100.0000% |

Notes:

1. The analyses presented in the table use a drainage path length of 1500 ft and slope of 0.5% for the final cover and a drainage path length of 1500 ft and slope of 1% for the drainage layer in the liner.
2. The proposed liner system (Base Case [top], run I_1A) is described in Table 1. The prescribed NYSDEC Part 360 liner is described in Table 5.
3. The efficiency of the liner is calculated using the formula:

$$Efficiency = 100\% - \frac{Leakage}{Precipitation} \cdot$$

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Figures

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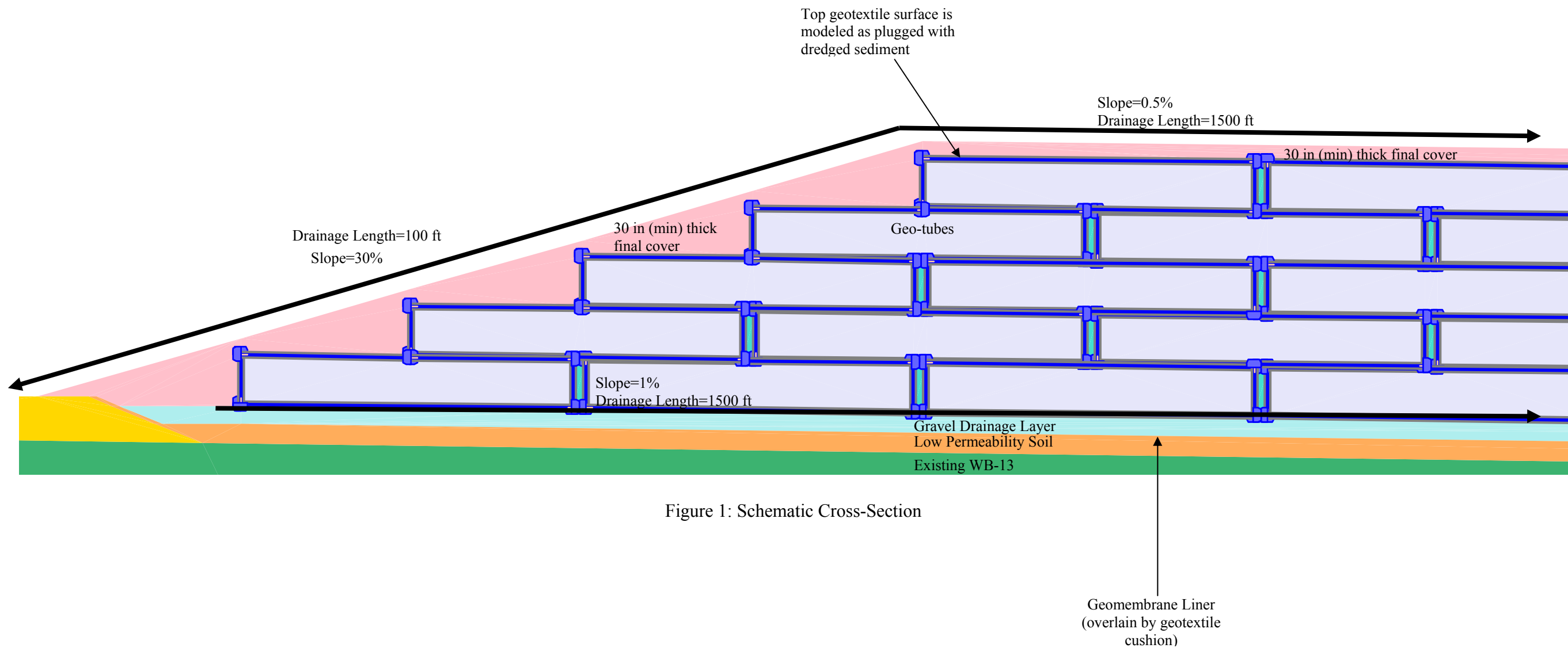


Figure 1: Schematic Cross-Section

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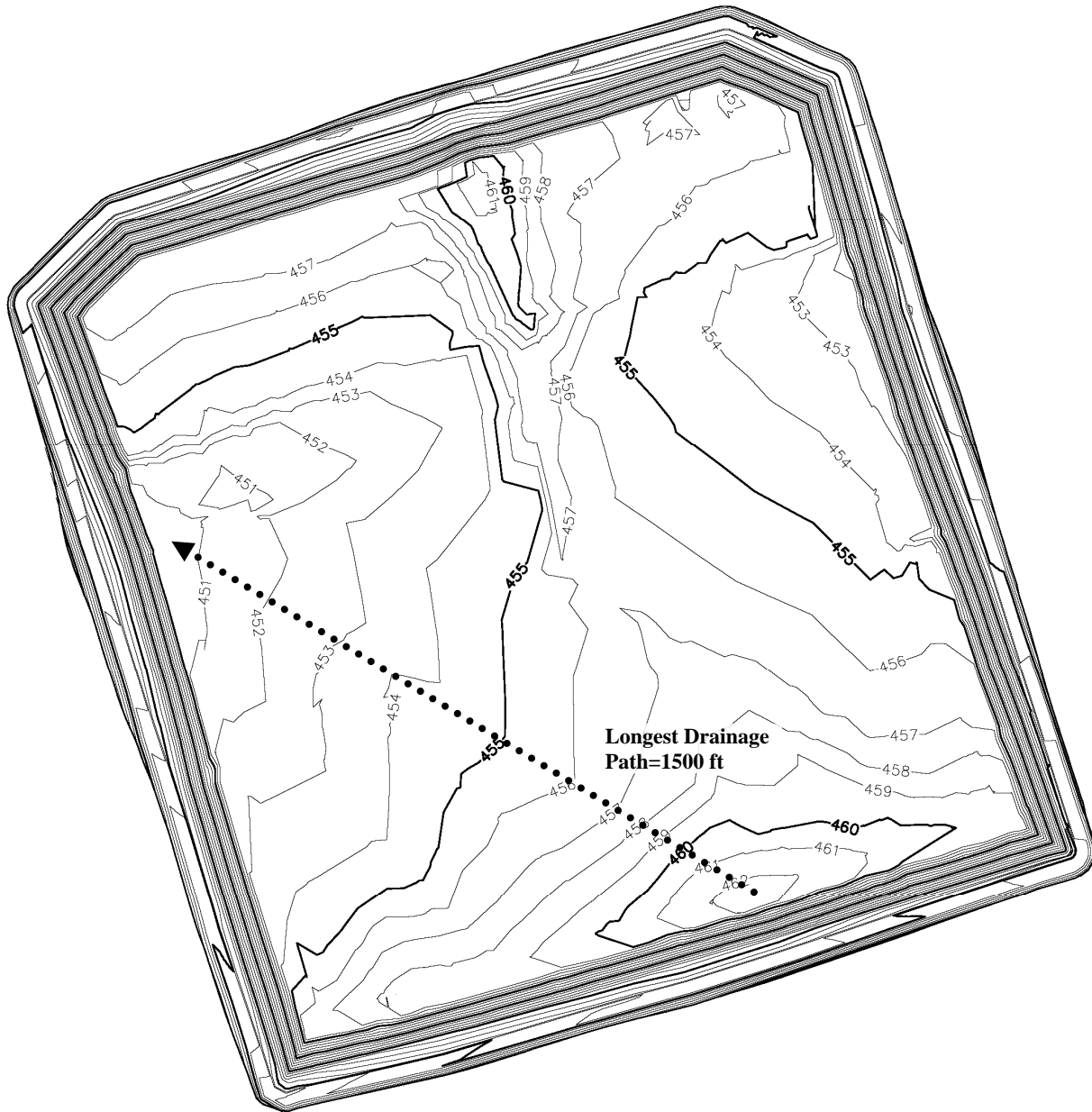


Figure 2: Design Top of Final Cover

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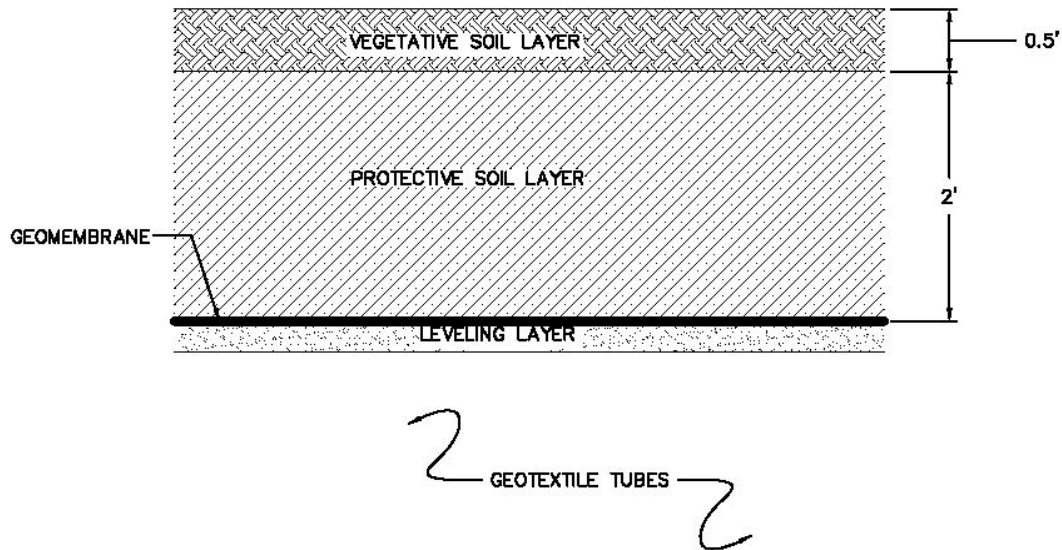


Figure 3A: Section View of Final Cover (SCA top)

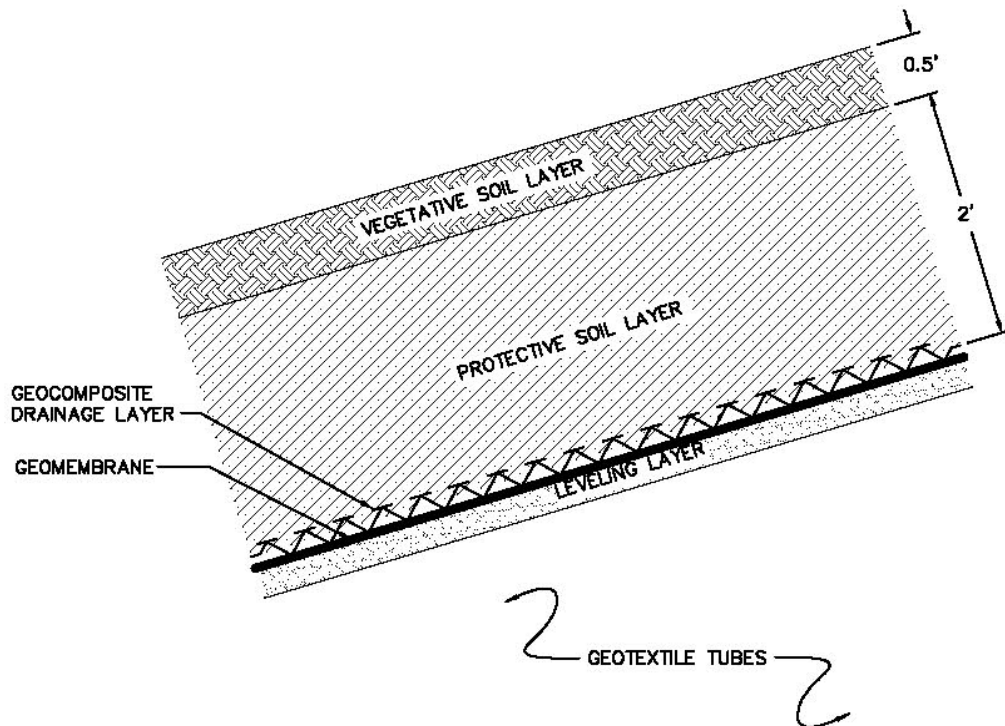
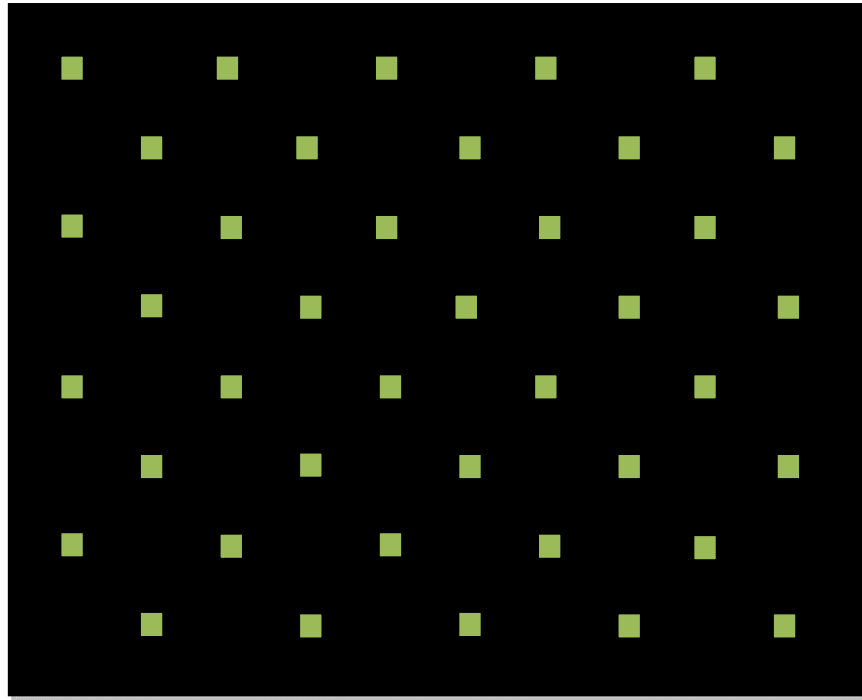


Figure 3B: Section View of Final Cover (SCA side slopes)

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K_{DREDGE} 
 K_{GT} 

Figure 4: Conceptual View of Geo-tube/Dredge Material Layer Equivalent Hydraulic Conductivity

Notes:

1. This figure illustrates the modeling of the geo-tube/dredge material layer equivalent hydraulic conductivity.
2. The open area of the geo-tubes (assumed to be 4% of total area) is covered with dredge filter cake material, as shown. Therefore, the hydraulic conductivity of the geo-tubes/dredge material interface layer was modeled as a liner with equivalent hydraulic conductivity.

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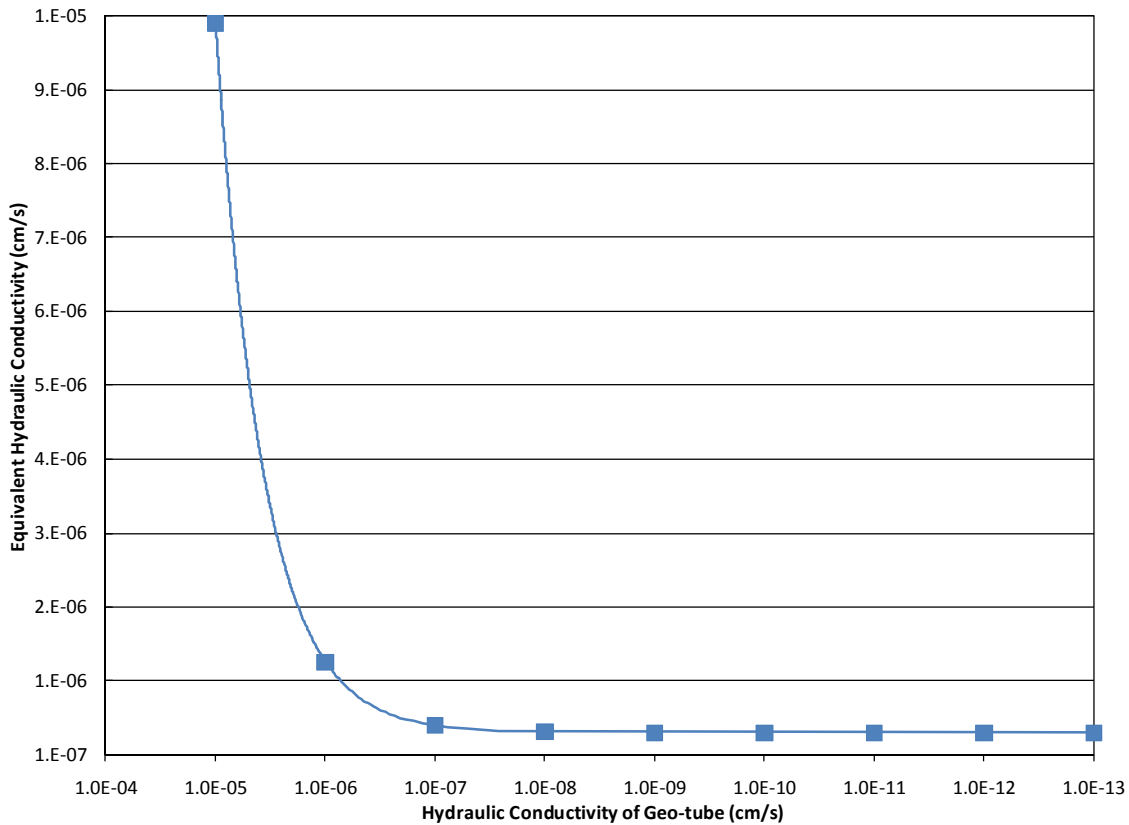


Figure 5: Sensitivity of Dredge Material/Geo-tube Equivalent Hydraulic Conductivity to the Hydraulic Conductivity of the Geo-tube Material

Notes:

1. This figure shows the equivalent hydraulic conductivity of Layer 5, the top of geo-tubes with open areas plugged with dredge material.
2. The actual fabric of a woven geotextile was assumed to have a hydraulic conductivity of 10^{-8} cm/s or less.
3. The calculations were performed using a hydraulic conductivity of 1.0×10^{-5} cm/s for the dredge material.
4. Based on the assumed $K_{GT} = 10^{-8}$ cm/s, a K_{EQ} value of 4×10^{-7} cm/s has been assumed for the analyses.

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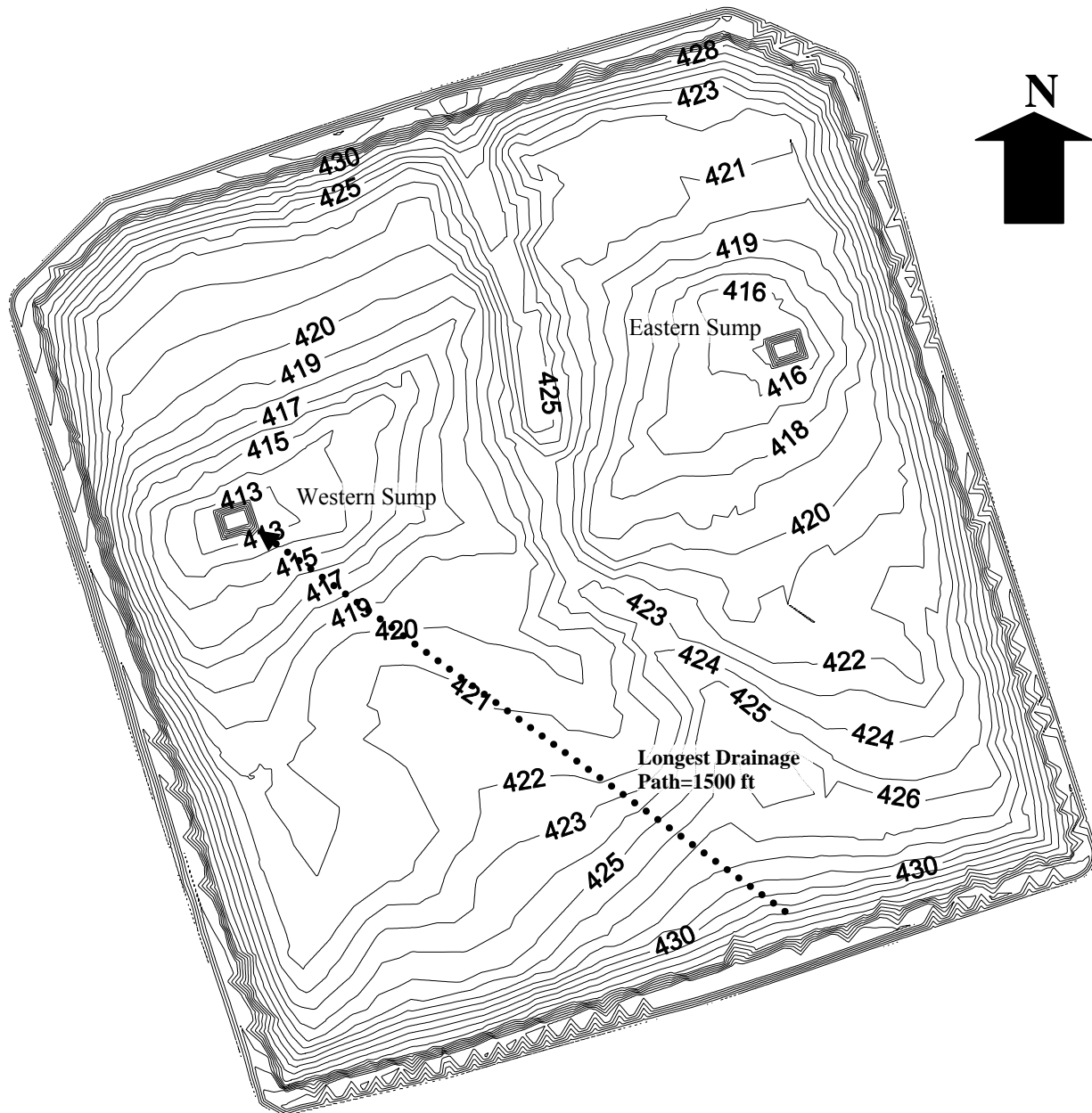


Figure 6: Top of Low Permeability Soil Liner Grades - 30 Years after Closure

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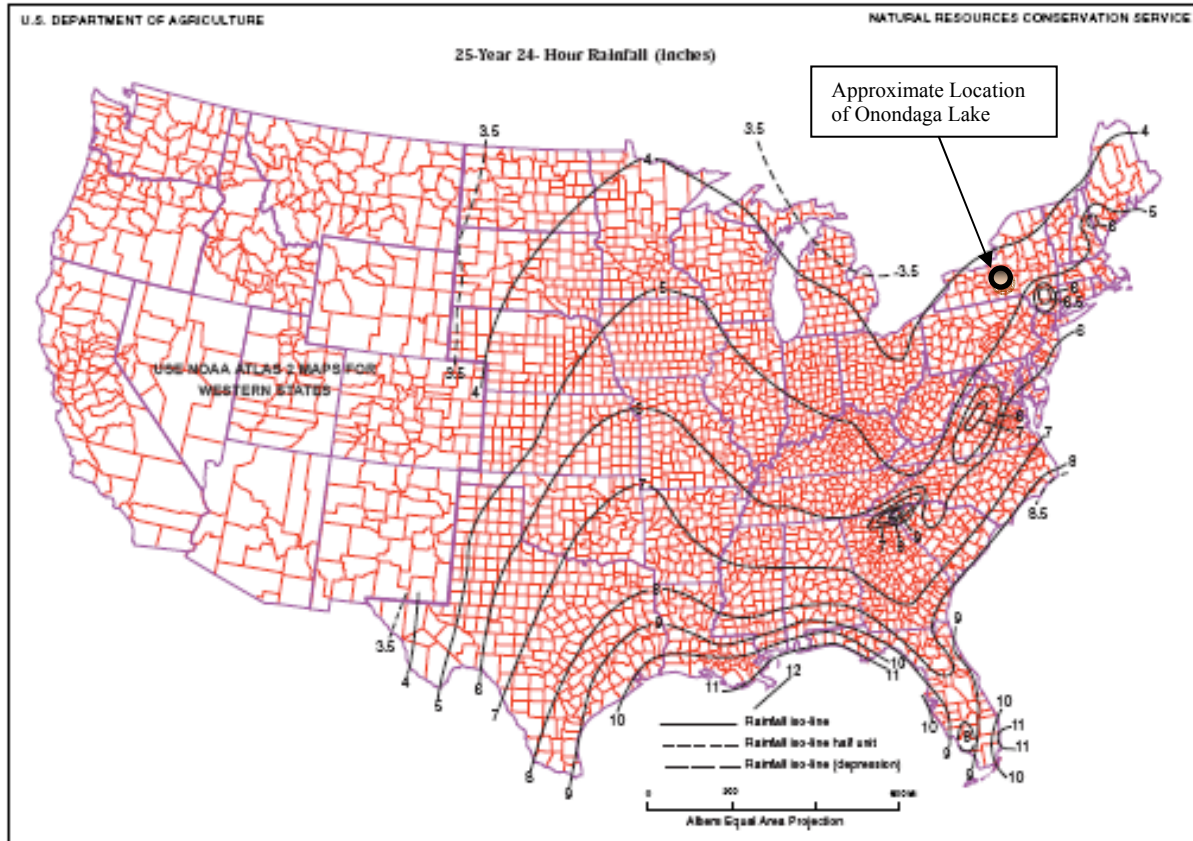


Figure 7: NRCS Map for Calculation of 25-year, 24-hour Storm Event [NRCS, 1986]

Note:

The value selected for the model is 4.40 inches, based on the approximate location of Onondaga Lake.

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Attachment 1: HELP Output Files

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Client: Honeywell Project: Onondaga Lake SCA Final Design Project/ Proposal No.: GJ4299 Task No.: 18

```
*****
*****
**
**
**
**      HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
**      HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
**      DEVELOPED BY ENVIRONMENTAL LABORATORY
**      USAE WATERWAYS EXPERIMENT STATION
**      FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
**
**
*****
*****
```

```
PRECIPITATION DATA FILE:  C:\PROGRA~1\HELP\OLPRECIP.D4
TEMPERATURE DATA FILE:   C:\PROGRA~1\HELP\OLTEMP.D7
SOLAR RADIATION DATA FILE: C:\PROGRA~1\HELP\OLSOLAR.D13
EVAPOTRANSPIRATION DATA: C:\PROGRA~1\HELP\OL_LAI35.D11
SOIL AND DESIGN DATA FILE: C:\PROGRA~1\HELP\I__1A.D10
OUTPUT DATA FILE:        C:\PROGRA~1\HELP\I__1A.OUT
```

TIME: 13:38 DATE: 12/ 9/2009

```
*****
TITLE: Onondaga Lake SCA - Final Design
*****
```

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER WERE
COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.

LAYER 1

```
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 108
THICKNESS           = 6.00 INCHES
POROSITY             = 0.4630 VOL/VOL
FIELD CAPACITY       = 0.2320 VOL/VOL
WILTING POINT        = 0.1160 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4586 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.99999975000E-04 CM/SEC
```

LAYER 2

TYPE 2 - LATERAL DRAINAGE LAYER

Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: **Honeywell** Project: **Onondaga Lake SCA Final Design** Project/ Proposal No.: **GJ4299** Task No.: **18**

MATERIAL TEXTURE NUMBER 113
THICKNESS = 24.00 INCHES
POROSITY = 0.4300 VOL/VOL
FIELD CAPACITY = 0.3210 VOL/VOL
WILTING POINT = 0.2210 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4310 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999975000E-05 CM/SEC
SLOPE = 0.51 PERCENT
DRAINAGE LENGTH = 1500.0 FEET

LAYER 3

TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 36
THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.399999993000E-12 CM/SEC
FML PINHOLE DENSITY = 0.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
FML PLACEMENT QUALITY = 3 - GOOD

LAYER 4

TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 221
THICKNESS = 6.00 INCHES
POROSITY = 0.3970 VOL/VOL
FIELD CAPACITY = 0.0320 VOL/VOL
WILTING POINT = 0.0130 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0320 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 1.0000000000 CM/SEC

LAYER 5

TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 120
THICKNESS = 0.06 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0100 VOL/VOL
WILTING POINT = 0.0050 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.8500 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.400000005000E-06 CM/SEC

LAYER 6

Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: Honeywell Project: Onondaga Lake SCA Final Design Project/ Proposal No.: GJ4299 Task No.: 18

TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 122
THICKNESS = 0.06 INCHES
POROSITY = 0.4190 VOL/VOL
FIELD CAPACITY = 0.3070 VOL/VOL
WILTING POINT = 0.1800 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.3070 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999975000E-05 CM/SEC

LAYER 7

TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 122
THICKNESS = 360.00 INCHES
POROSITY = 0.4190 VOL/VOL
FIELD CAPACITY = 0.3070 VOL/VOL
WILTING POINT = 0.1800 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4190 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999975000E-05 CM/SEC

LAYER 8

TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 121
THICKNESS = 12.00 INCHES
POROSITY = 0.3970 VOL/VOL
FIELD CAPACITY = 0.0320 VOL/VOL
WILTING POINT = 0.0130 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0320 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 10.0000000000 CM/SEC
SLOPE = 1.00 PERCENT
DRAINAGE LENGTH = 1500.0 FEET

LAYER 9

TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 36
THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.399999993000E-12 CM/SEC
FML PINHOLE DENSITY = 0.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
FML PLACEMENT QUALITY = 3 - GOOD

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LAYER 10

TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 124

| | | | |
|----------------------------|---|--------------------|---------|
| THICKNESS | = | 6.00 | INCHES |
| POROSITY | = | 0.3650 | VOL/VOL |
| FIELD CAPACITY | = | 0.3050 | VOL/VOL |
| WILTING POINT | = | 0.2020 | VOL/VOL |
| INITIAL SOIL WATER CONTENT | = | 0.3650 | VOL/VOL |
| EFFECTIVE SAT. HYD. COND. | = | 0.999999997000E-06 | CM/SEC |

LAYER 11

TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 113

| | | | |
|----------------------------|---|--------------------|---------|
| THICKNESS | = | 6.00 | INCHES |
| POROSITY | = | 0.4300 | VOL/VOL |
| FIELD CAPACITY | = | 0.3210 | VOL/VOL |
| WILTING POINT | = | 0.2210 | VOL/VOL |
| INITIAL SOIL WATER CONTENT | = | 0.3049 | VOL/VOL |
| EFFECTIVE SAT. HYD. COND. | = | 0.999999975000E-05 | CM/SEC |

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
SOIL DATA BASE USING SOIL TEXTURE # 8 WITH A
GOOD STAND OF GRASS, A SURFACE SLOPE OF 1.1%
AND A SLOPE LENGTH OF 1500. FEET.

| | | | |
|------------------------------------|---|---------|-------------|
| SCS RUNOFF CURVE NUMBER | = | 68.20 | |
| FRACTION OF AREA ALLOWING RUNOFF | = | 100.0 | PERCENT |
| AREA PROJECTED ON HORIZONTAL PLANE | = | 1.000 | ACRES |
| EVAPORATIVE ZONE DEPTH | = | 30.0 | INCHES |
| INITIAL WATER IN EVAPORATIVE ZONE | = | 13.096 | INCHES |
| UPPER LIMIT OF EVAPORATIVE STORAGE | = | 13.098 | INCHES |
| LOWER LIMIT OF EVAPORATIVE STORAGE | = | 6.000 | INCHES |
| INITIAL SNOW WATER | = | 0.000 | INCHES |
| INITIAL WATER IN LAYER MATERIALS | = | 168.601 | INCHES |
| TOTAL INITIAL WATER | = | 168.601 | INCHES |
| TOTAL SUBSURFACE INFLOW | = | 0.00 | INCHES/YEAR |

EVAPOTRANSPIRATION AND WEATHER DATA

NOTE: EVAPOTRANSPIRATION DATA WAS OBTAINED FROM
SYRACUSE NEW YORK

Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: **Honeywell** Project: **Onondaga Lake SCA Final Design** Project/ Proposal No.: **GJ4299** Task No.: **18**

STATION LATITUDE = 43.07 DEGREES
 MAXIMUM LEAF AREA INDEX = 3.50
 START OF GROWING SEASON (JULIAN DATE) = 124
 END OF GROWING SEASON (JULIAN DATE) = 284
 EVAPORATIVE ZONE DEPTH = 30.0 INCHES
 AVERAGE ANNUAL WIND SPEED = 9.70 MPH
 AVERAGE 1ST QUARTER RELATIVE HUMIDITY = 72.00 %
 AVERAGE 2ND QUARTER RELATIVE HUMIDITY = 68.00 %
 AVERAGE 3RD QUARTER RELATIVE HUMIDITY = 75.00 %
 AVERAGE 4TH QUARTER RELATIVE HUMIDITY = 76.00 %

NOTE: PRECIPITATION DATA WAS SYNTHETICALLY GENERATED USING
 COEFFICIENTS FOR SYRACUSE NEW YORK

NORMAL MEAN MONTHLY PRECIPITATION (INCHES)

| JAN/JUL | FEB/AUG | MAR/SEP | APR/OCT | MAY/NOV | JUN/DEC |
|---------|---------|---------|---------|---------|---------|
| 2.61 | 2.65 | 3.11 | 3.34 | 3.16 | 3.63 |
| 3.76 | 3.77 | 3.29 | 3.14 | 3.45 | 3.20 |

NOTE: TEMPERATURE DATA WAS SYNTHETICALLY GENERATED USING
 COEFFICIENTS FOR SYRACUSE NEW YORK

NORMAL MEAN MONTHLY TEMPERATURE (DEGREES FAHRENHEIT)

| JAN/JUL | FEB/AUG | MAR/SEP | APR/OCT | MAY/NOV | JUN/DEC |
|---------|---------|---------|---------|---------|---------|
| 22.80 | 24.00 | 33.30 | 46.10 | 57.00 | 66.30 |
| 70.90 | 69.30 | 62.10 | 51.30 | 40.60 | 28.30 |

NOTE: SOLAR RADIATION DATA WAS SYNTHETICALLY GENERATED USING
 COEFFICIENTS FOR SYRACUSE NEW YORK
 AND STATION LATITUDE = 43.07 DEGREES

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100

| | JAN/JUL | FEB/AUG | MAR/SEP | APR/OCT | MAY/NOV | JUN/DEC |
|-----------------|---------|---------|---------|---------|---------|---------|
| PRECIPITATION | | | | | | |
| TOTALS | 2.59 | 2.72 | 3.16 | 3.27 | 3.09 | 3.71 |
| | 3.87 | 3.95 | 3.27 | 2.95 | 3.40 | 3.22 |
| STD. DEVIATIONS | 0.70 | 0.96 | 1.19 | 1.19 | 1.31 | 1.57 |
| | 1.67 | 1.76 | 1.60 | 1.16 | 1.19 | 0.76 |

RUNOFF

Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: **Honeywell** Project: **Onondaga Lake SCA Final Design** Project/ Proposal No.: **GJ4299** Task No.: **18**

| | | | | | | |
|---|--------|--------|--------|--------|--------|--------|
| ----- | | | | | | |
| TOTALS | 0.840 | 1.708 | 5.121 | 2.038 | 0.493 | 0.312 |
| | 0.056 | 0.019 | 0.027 | 0.158 | 0.770 | 1.251 |
| STD. DEVIATIONS | 0.941 | 1.678 | 2.481 | 1.397 | 0.690 | 0.666 |
| | 0.419 | 0.152 | 0.165 | 0.473 | 1.120 | 1.084 |
| EVAPOTRANSPIRATION | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.491 | 0.407 | 0.480 | 1.806 | 2.889 | 5.480 |
| | 6.166 | 4.126 | 2.211 | 1.045 | 0.710 | 0.497 |
| STD. DEVIATIONS | 0.080 | 0.077 | 0.169 | 0.691 | 0.799 | 0.644 |
| | 0.755 | 1.297 | 0.674 | 0.154 | 0.135 | 0.109 |
| LATERAL DRAINAGE COLLECTED FROM LAYER 2 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0000 | 0.0000 | 0.0000 | 0.0002 | 0.0003 | 0.0002 |
| | 0.0001 | 0.0000 | 0.0000 | 0.0001 | 0.0002 | 0.0002 |
| STD. DEVIATIONS | 0.0000 | 0.0000 | 0.0000 | 0.0001 | 0.0000 | 0.0000 |
| | 0.0000 | 0.0000 | 0.0001 | 0.0001 | 0.0002 | 0.0002 |
| PERCOLATION/LEAKAGE THROUGH LAYER 3 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0052 | 0.0043 | 0.0052 | 0.0129 | 0.0194 | 0.0150 |
| | 0.0069 | 0.0024 | 0.0021 | 0.0046 | 0.0100 | 0.0128 |
| STD. DEVIATIONS | 0.0019 | 0.0011 | 0.0018 | 0.0046 | 0.0006 | 0.0014 |
| | 0.0034 | 0.0037 | 0.0044 | 0.0071 | 0.0084 | 0.0065 |
| PERCOLATION/LEAKAGE THROUGH LAYER 5 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0052 | 0.0043 | 0.0052 | 0.0129 | 0.0194 | 0.0150 |
| | 0.0069 | 0.0024 | 0.0021 | 0.0046 | 0.0100 | 0.0128 |
| STD. DEVIATIONS | 0.0019 | 0.0011 | 0.0018 | 0.0046 | 0.0006 | 0.0014 |
| | 0.0034 | 0.0037 | 0.0044 | 0.0071 | 0.0084 | 0.0065 |
| PERCOLATION/LEAKAGE THROUGH LAYER 7 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0052 | 0.0043 | 0.0052 | 0.0129 | 0.0194 | 0.0150 |
| | 0.0069 | 0.0024 | 0.0021 | 0.0046 | 0.0100 | 0.0128 |
| STD. DEVIATIONS | 0.0019 | 0.0011 | 0.0018 | 0.0046 | 0.0006 | 0.0014 |
| | 0.0034 | 0.0037 | 0.0044 | 0.0071 | 0.0084 | 0.0065 |
| LATERAL DRAINAGE COLLECTED FROM LAYER 8 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0052 | 0.0043 | 0.0051 | 0.0125 | 0.0194 | 0.0153 |
| | 0.0071 | 0.0024 | 0.0020 | 0.0045 | 0.0097 | 0.0131 |
| STD. DEVIATIONS | 0.0020 | 0.0011 | 0.0017 | 0.0047 | 0.0006 | 0.0014 |
| | 0.0034 | 0.0037 | 0.0044 | 0.0070 | 0.0084 | 0.0066 |
| PERCOLATION/LEAKAGE THROUGH LAYER 10 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 |
| | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 |
| STD. DEVIATIONS | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 |
| | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 |

Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: **Honeywell** Project: **Onondaga Lake SCA Final Design** Project/ Proposal No.: **GJ4299** Task No.: **18**

PERCOLATION/LEAKAGE THROUGH LAYER 11

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| TOTALS | 0.0004 | 0.0004 | 0.0004 | 0.0004 | 0.0004 | 0.0003 |
| | 0.0003 | 0.0001 | 0.0001 | 0.0001 | 0.0002 | 0.0003 |
| STD. DEVIATIONS | 0.0011 | 0.0009 | 0.0009 | 0.0008 | 0.0008 | 0.0007 |
| | 0.0006 | 0.0003 | 0.0003 | 0.0003 | 0.0006 | 0.0006 |

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3

| | | | | | | |
|-----------------|--------|--------|--------|---------|---------|---------|
| AVERAGES | 5.1671 | 4.5585 | 5.1793 | 17.5567 | 26.9031 | 21.4400 |
| | 8.4427 | 2.6768 | 2.6671 | 6.0991 | 13.9574 | 16.9021 |
| STD. DEVIATIONS | 2.5364 | 1.2677 | 2.2985 | 7.1492 | 0.7459 | 2.1231 |
| | 5.0019 | 4.8488 | 6.1433 | 9.8299 | 12.0795 | 9.2454 |

DAILY AVERAGE HEAD ON TOP OF LAYER 5

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| AVERAGES | 0.0001 | 0.0001 | 0.0001 | 0.0001 | 0.0002 | 0.0002 |
| | 0.0001 | 0.0000 | 0.0000 | 0.0001 | 0.0001 | 0.0001 |
| STD. DEVIATIONS | 0.0000 | 0.0000 | 0.0000 | 0.0001 | 0.0000 | 0.0000 |
| | 0.0000 | 0.0000 | 0.0001 | 0.0001 | 0.0001 | 0.0001 |

DAILY AVERAGE HEAD ON TOP OF LAYER 7

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| AVERAGES | 0.0002 | 0.0002 | 0.0002 | 0.0005 | 0.0007 | 0.0006 |
| | 0.0002 | 0.0001 | 0.0001 | 0.0002 | 0.0004 | 0.0005 |
| STD. DEVIATIONS | 0.0001 | 0.0000 | 0.0001 | 0.0002 | 0.0000 | 0.0001 |
| | 0.0001 | 0.0001 | 0.0002 | 0.0003 | 0.0003 | 0.0002 |

DAILY AVERAGE HEAD ON TOP OF LAYER 9

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| AVERAGES | 0.0004 | 0.0004 | 0.0004 | 0.0011 | 0.0017 | 0.0013 |
| | 0.0006 | 0.0002 | 0.0002 | 0.0004 | 0.0009 | 0.0011 |
| STD. DEVIATIONS | 0.0002 | 0.0001 | 0.0001 | 0.0004 | 0.0000 | 0.0001 |
| | 0.0003 | 0.0003 | 0.0004 | 0.0006 | 0.0007 | 0.0006 |

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

| | INCHES | | CU. FEET | PERCENT |
|--------------------|--------|-----------|----------|---------|
| PRECIPITATION | 39.20 | (4.823) | 142279.7 | 100.00 |
| RUNOFF | 12.794 | (3.3907) | 46441.28 | 32.641 |
| EVAPOTRANSPIRATION | 26.309 | (2.2364) | 95501.46 | 67.122 |

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| | | | |
|--|--------------------|---------|---------|
| LATERAL DRAINAGE COLLECTED FROM LAYER 2 | 0.00145 (0.00054) | 5.246 | 0.00369 |
| PERCOLATION/LEAKAGE THROUGH LAYER 3 | 0.10082 (0.02666) | 365.966 | 0.25722 |
| AVERAGE HEAD ON TOP OF LAYER 3 | 10.962 (3.116) | | |
| PERCOLATION/LEAKAGE THROUGH LAYER 5 | 0.10082 (0.02666) | 365.966 | 0.25722 |
| AVERAGE HEAD ON TOP OF LAYER 5 | 0.000 (0.000) | | |
| PERCOLATION/LEAKAGE THROUGH LAYER 7 | 0.10082 (0.02666) | 365.966 | 0.25722 |
| AVERAGE HEAD ON TOP OF LAYER 7 | 0.000 (0.000) | | |
| LATERAL DRAINAGE COLLECTED FROM LAYER 8 | 0.10081 (0.02665) | 365.952 | 0.25721 |
| PERCOLATION/LEAKAGE THROUGH LAYER 10 | 0.00001 (0.00000) | 0.020 | 0.00001 |
| AVERAGE HEAD ON TOP OF LAYER 9 | 0.001 (0.000) | | |
| PERCOLATION/LEAKAGE THROUGH LAYER 11 | 0.00340 (0.00747) | 12.332 | 0.00867 |
| CHANGE IN WATER STORAGE | -0.013 (1.5171) | -46.59 | -0.033 |

| PEAK DAILY VALUES FOR YEARS | 1 THROUGH | 100 |
|--|------------|------------|
| | (INCHES) | (CU. FT.) |
| PRECIPITATION | 4.40 | 15972.000 |
| RUNOFF | 4.342 | 15759.7656 |
| DRAINAGE COLLECTED FROM LAYER 2 | 0.00002 | 0.06081 |
| PERCOLATION/LEAKAGE THROUGH LAYER 3 | 0.000703 | 2.55133 |
| AVERAGE HEAD ON TOP OF LAYER 3 | 30.000 | |
| MAXIMUM HEAD ON TOP OF LAYER 3 | 37.397 | |
| LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN) | 596.4 FEET | |
| PERCOLATION/LEAKAGE THROUGH LAYER 5 | 0.000703 | 2.55133 |
| AVERAGE HEAD ON TOP OF LAYER 5 | 0.000 | |

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| | | |
|--|----------|------------|
| PERCOLATION/LEAKAGE THROUGH LAYER 7 | 0.000703 | 2.55133 |
| AVERAGE HEAD ON TOP OF LAYER 7 | 0.001 | |
| DRAINAGE COLLECTED FROM LAYER 8 | 0.00070 | 2.55124 |
| PERCOLATION/LEAKAGE THROUGH LAYER 10 | 0.000000 | 0.00009 |
| AVERAGE HEAD ON TOP OF LAYER 9 | 0.002 | |
| MAXIMUM HEAD ON TOP OF LAYER 9 | 0.004 | |
| LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN) | 0.0 FEET | |
| PERCOLATION/LEAKAGE THROUGH LAYER 11 | 0.000314 | 1.14110 |
| SNOW WATER | 9.62 | 34911.7891 |
| MAXIMUM VEG. SOIL WATER (VOL/VOL) | 0.4366 | |
| MINIMUM VEG. SOIL WATER (VOL/VOL) | 0.2000 | |

*** Maximum heads are computed using McEnroe's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner
by Bruce M. McEnroe, University of Kansas
ASCE Journal of Environmental Engineering
Vol. 119, No. 2, March 1993, pp. 262-270.

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FINAL WATER STORAGE AT END OF YEAR 100

| LAYER | (INCHES) | (VOL/ VOL) |
|------------|------------|--------------|
| 1 | 2.7516 | 0.4586 |
| 2 | 9.3544 | 0.3898 |
| 3 | 0.0000 | 0.0000 |
| 4 | 0.1920 | 0.0320 |
| 5 | 0.0510 | 0.8500 |
| 6 | 0.0184 | 0.3070 |
| 7 | 150.8400 | 0.4190 |
| 8 | 0.3840 | 0.0320 |
| 9 | 0.0000 | 0.0000 |
| 10 | 2.1900 | 0.3650 |
| 11 | 1.4902 | 0.2484 |
| SNOW WATER | 0.046 | |

Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: Honeywell Project: Onondaga Lake SCA Final Design Project/ Proposal No.: GJ4299 Task No.: 18

```
*****
*****
**
**
**
**      HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
**      HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
**      DEVELOPED BY ENVIRONMENTAL LABORATORY
**      USAE WATERWAYS EXPERIMENT STATION
**      FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
**
**
*****
*****
```

```
PRECIPITATION DATA FILE: C:\PROGRA~1\HELP\OLPRECIP.D4
TEMPERATURE DATA FILE:  C:\PROGRA~1\HELP\OLTEMP.D7
SOLAR RADIATION DATA FILE: C:\PROGRA~1\HELP\OLSOLAR.D13
EVAPOTRANSPIRATION DATA:  C:\PROGRA~1\HELP\OL_LAI35.D11
SOIL AND DESIGN DATA FILE: C:\PROGRA~1\HELP\S_IIIIA.D10
OUTPUT DATA FILE:        C:\PROGRA~1\HELP\S_IIIIA.OUT
```

TIME: 13:47 DATE: 12/ 9/2009

```
*****
TITLE: Onondaga Lake SCA - Final Design
*****
```

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER WERE
COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.

LAYER 1

```
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 108
THICKNESS           = 6.00 INCHES
POROSITY             = 0.4630 VOL/VOL
FIELD CAPACITY       = 0.2320 VOL/VOL
WILTING POINT        = 0.1160 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4567 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.99999975000E-04 CM/SEC
```

LAYER 2

TYPE 2 - LATERAL DRAINAGE LAYER

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Client: **Honeywell** Project: **Onondaga Lake SCA Final Design** Project/ Proposal No.: **GJ4299** Task No.: **18**

MATERIAL TEXTURE NUMBER 113
THICKNESS = 24.00 INCHES
POROSITY = 0.4300 VOL/VOL
FIELD CAPACITY = 0.3210 VOL/VOL
WILTING POINT = 0.2210 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4057 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999975000E-05 CM/SEC

LAYER 3

TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 220
THICKNESS = 0.20 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0100 VOL/VOL
WILTING POINT = 0.0050 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 1.33000004000 CM/SEC
SLOPE = 30.00 PERCENT
DRAINAGE LENGTH = 100.0 FEET

LAYER 4

TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 36
THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.399999993000E-12 CM/SEC
FML PINHOLE DENSITY = 0.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
FML PLACEMENT QUALITY = 3 - GOOD

LAYER 5

TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 221
THICKNESS = 6.00 INCHES
POROSITY = 0.3970 VOL/VOL
FIELD CAPACITY = 0.0320 VOL/VOL
WILTING POINT = 0.0130 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0320 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 1.00000000000 CM/SEC

LAYER 6

Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: Honeywell Project: Onondaga Lake SCA Final Design Project/ Proposal No.: GJ4299 Task No.: 18

TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 120

THICKNESS = 0.06 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0100 VOL/VOL
WILTING POINT = 0.0050 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.8500 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.400000005000E-06 CM/SEC

LAYER 7

TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 122

THICKNESS = 0.06 INCHES
POROSITY = 0.4190 VOL/VOL
FIELD CAPACITY = 0.3070 VOL/VOL
WILTING POINT = 0.1800 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.3070 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999975000E-05 CM/SEC

LAYER 8

TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 122

THICKNESS = 360.00 INCHES
POROSITY = 0.4190 VOL/VOL
FIELD CAPACITY = 0.3070 VOL/VOL
WILTING POINT = 0.1800 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4190 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999975000E-05 CM/SEC

LAYER 9

TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 121

THICKNESS = 12.00 INCHES
POROSITY = 0.3970 VOL/VOL
FIELD CAPACITY = 0.0320 VOL/VOL
WILTING POINT = 0.0130 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0320 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 10.0000000000 CM/SEC
SLOPE = 1.00 PERCENT
DRAINAGE LENGTH = 1500.0 FEET

LAYER 10

Written by: Joseph Sura Date: 11/23/2009 Reviewed by: Fan Zhu / R. Kulasingam Date: 12/2/2009

Client: Honeywell Project: Onondaga Lake SCA Final Design Project/ Proposal No.: GJ4299 Task No.: 18

TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 36

| | | | |
|----------------------------|---|--------------------|------------|
| THICKNESS | = | 0.06 | INCHES |
| POROSITY | = | 0.0000 | VOL/VOL |
| FIELD CAPACITY | = | 0.0000 | VOL/VOL |
| WILTING POINT | = | 0.0000 | VOL/VOL |
| INITIAL SOIL WATER CONTENT | = | 0.0000 | VOL/VOL |
| EFFECTIVE SAT. HYD. COND. | = | 0.399999993000E-12 | CM/SEC |
| FML PINHOLE DENSITY | = | 0.00 | HOLES/ACRE |
| FML INSTALLATION DEFECTS | = | 1.00 | HOLES/ACRE |
| FML PLACEMENT QUALITY | = | 3 | - GOOD |

LAYER 11

TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 124

| | | | |
|----------------------------|---|--------------------|---------|
| THICKNESS | = | 6.00 | INCHES |
| POROSITY | = | 0.3650 | VOL/VOL |
| FIELD CAPACITY | = | 0.3050 | VOL/VOL |
| WILTING POINT | = | 0.2020 | VOL/VOL |
| INITIAL SOIL WATER CONTENT | = | 0.3650 | VOL/VOL |
| EFFECTIVE SAT. HYD. COND. | = | 0.999999997000E-06 | CM/SEC |

LAYER 12

TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 113

| | | | |
|----------------------------|---|--------------------|---------|
| THICKNESS | = | 6.00 | INCHES |
| POROSITY | = | 0.4300 | VOL/VOL |
| FIELD CAPACITY | = | 0.3210 | VOL/VOL |
| WILTING POINT | = | 0.2210 | VOL/VOL |
| INITIAL SOIL WATER CONTENT | = | 0.3038 | VOL/VOL |
| EFFECTIVE SAT. HYD. COND. | = | 0.999999975000E-05 | CM/SEC |

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
SOIL DATA BASE USING SOIL TEXTURE # 8 WITH A
GOOD STAND OF GRASS, A SURFACE SLOPE OF 30.0%
AND A SLOPE LENGTH OF 100. FEET.

| | | | |
|------------------------------------|---|--------|---------|
| SCS RUNOFF CURVE NUMBER | = | 76.10 | |
| FRACTION OF AREA ALLOWING RUNOFF | = | 100.0 | PERCENT |
| AREA PROJECTED ON HORIZONTAL PLANE | = | 1.000 | ACRES |
| EVAPORATIVE ZONE DEPTH | = | 30.0 | INCHES |
| INITIAL WATER IN EVAPORATIVE ZONE | = | 12.477 | INCHES |
| UPPER LIMIT OF EVAPORATIVE STORAGE | = | 13.098 | INCHES |

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| | | | |
|------------------------------------|---|---------|-------------|
| LOWER LIMIT OF EVAPORATIVE STORAGE | = | 6.000 | INCHES |
| INITIAL SNOW WATER | = | 0.000 | INCHES |
| INITIAL WATER IN LAYER MATERIALS | = | 167.977 | INCHES |
| TOTAL INITIAL WATER | = | 167.977 | INCHES |
| TOTAL SUBSURFACE INFLOW | = | 0.00 | INCHES/YEAR |

EVAPOTRANSPIRATION AND WEATHER DATA

NOTE: EVAPOTRANSPIRATION DATA WAS OBTAINED FROM
SYRACUSE NEW YORK

| | | | |
|---------------------------------------|---|-------|---------|
| STATION LATITUDE | = | 43.07 | DEGREES |
| MAXIMUM LEAF AREA INDEX | = | 3.50 | |
| START OF GROWING SEASON (JULIAN DATE) | = | 124 | |
| END OF GROWING SEASON (JULIAN DATE) | = | 284 | |
| EVAPORATIVE ZONE DEPTH | = | 30.0 | INCHES |
| AVERAGE ANNUAL WIND SPEED | = | 9.70 | MPH |
| AVERAGE 1ST QUARTER RELATIVE HUMIDITY | = | 72.00 | % |
| AVERAGE 2ND QUARTER RELATIVE HUMIDITY | = | 68.00 | % |
| AVERAGE 3RD QUARTER RELATIVE HUMIDITY | = | 75.00 | % |
| AVERAGE 4TH QUARTER RELATIVE HUMIDITY | = | 76.00 | % |

NOTE: PRECIPITATION DATA WAS SYNTHETICALLY GENERATED USING
COEFFICIENTS FOR SYRACUSE NEW YORK

NORMAL MEAN MONTHLY PRECIPITATION (INCHES)

| JAN/JUL | FEB/AUG | MAR/SEP | APR/OCT | MAY/NOV | JUN/DEC |
|---------|---------|---------|---------|---------|---------|
| 2.61 | 2.65 | 3.11 | 3.34 | 3.16 | 3.63 |
| 3.76 | 3.77 | 3.29 | 3.14 | 3.45 | 3.20 |

NOTE: TEMPERATURE DATA WAS SYNTHETICALLY GENERATED USING
COEFFICIENTS FOR SYRACUSE NEW YORK

NORMAL MEAN MONTHLY TEMPERATURE (DEGREES FAHRENHEIT)

| JAN/JUL | FEB/AUG | MAR/SEP | APR/OCT | MAY/NOV | JUN/DEC |
|---------|---------|---------|---------|---------|---------|
| 22.80 | 24.00 | 33.30 | 46.10 | 57.00 | 66.30 |
| 70.90 | 69.30 | 62.10 | 51.30 | 40.60 | 28.30 |

NOTE: SOLAR RADIATION DATA WAS SYNTHETICALLY GENERATED USING
COEFFICIENTS FOR SYRACUSE NEW YORK
AND STATION LATITUDE = 43.07 DEGREES

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| AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100 | | | | | | |
|--|------------------|------------------|------------------|------------------|------------------|------------------|
| | JAN/JUL | FEB/AUG | MAR/SEP | APR/OCT | MAY/NOV | JUN/DEC |
| ----- | | | | | | |
| PRECIPITATION | | | | | | |
| ----- | | | | | | |
| TOTALS | 2.59 3.87 | 2.72 3.95 | 3.16 3.27 | 3.27 2.95 | 3.09 3.40 | 3.71 3.22 |
| STD. DEVIATIONS | 0.70 1.67 | 0.96 1.76 | 1.19 1.60 | 1.19 1.16 | 1.31 1.19 | 1.57 0.76 |
| RUNOFF | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.497 0.038 | 1.349 0.037 | 4.738 0.071 | 1.504 0.048 | 0.063 0.080 | 0.094 0.259 |
| STD. DEVIATIONS | 0.697 0.224 | 1.446 0.147 | 2.349 0.214 | 1.452 0.155 | 0.208 0.213 | 0.310 0.487 |
| EVAPOTRANSPIRATION | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.491 4.723 | 0.407 3.571 | 0.479 2.156 | 1.824 1.070 | 2.916 0.725 | 5.443 0.498 |
| STD. DEVIATIONS | 0.080 1.357 | 0.077 1.206 | 0.168 0.670 | 0.702 0.183 | 0.812 0.141 | 0.668 0.111 |
| LATERAL DRAINAGE COLLECTED FROM LAYER 3 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0459 0.0782 | 0.0000 0.0232 | 0.1598 0.0449 | 1.8128 0.3348 | 0.8908 1.0917 | 0.3306 1.1946 |
| STD. DEVIATIONS | 0.1518 0.1773 | 0.0000 0.1174 | 0.4555 0.1629 | 0.7230 0.6606 | 0.6554 1.0613 | 0.3481 0.9642 |
| PERCOLATION/LEAKAGE THROUGH LAYER 4 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0008 0.0025 | 0.0000 0.0006 | 0.0017 0.0013 | 0.0247 0.0063 | 0.0222 0.0183 | 0.0121 0.0205 |
| STD. DEVIATIONS | 0.0029 0.0041 | 0.0000 0.0028 | 0.0051 0.0043 | 0.0103 0.0115 | 0.0076 0.0156 | 0.0059 0.0136 |
| PERCOLATION/LEAKAGE THROUGH LAYER 6 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0008 0.0025 | 0.0000 0.0006 | 0.0017 0.0013 | 0.0247 0.0063 | 0.0222 0.0183 | 0.0121 0.0205 |
| STD. DEVIATIONS | 0.0029 0.0041 | 0.0000 0.0028 | 0.0051 0.0043 | 0.0103 0.0115 | 0.0076 0.0156 | 0.0059 0.0136 |
| PERCOLATION/LEAKAGE THROUGH LAYER 8 | | | | | | |
| ----- | | | | | | |
| TOTALS | 0.0008 0.0025 | 0.0000 0.0006 | 0.0017 0.0013 | 0.0247 0.0063 | 0.0222 0.0183 | 0.0121 0.0205 |
| STD. DEVIATIONS | 0.0029 0.0041 | 0.0000 0.0028 | 0.0051 0.0043 | 0.0103 0.0115 | 0.0076 0.0156 | 0.0059 0.0136 |

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LATERAL DRAINAGE COLLECTED FROM LAYER 9

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| TOTALS | 0.0010 | 0.0000 | 0.0015 | 0.0238 | 0.0228 | 0.0123 |
| | 0.0027 | 0.0006 | 0.0013 | 0.0061 | 0.0177 | 0.0213 |
| STD. DEVIATIONS | 0.0033 | 0.0000 | 0.0046 | 0.0108 | 0.0078 | 0.0059 |
| | 0.0042 | 0.0028 | 0.0043 | 0.0112 | 0.0155 | 0.0137 |

PERCOLATION/LEAKAGE THROUGH LAYER 11

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| TOTALS | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 |
| | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 |
| STD. DEVIATIONS | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 |
| | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 | 0.0000 |

PERCOLATION/LEAKAGE THROUGH LAYER 12

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| TOTALS | 0.0000 | 0.0000 | 0.0000 | 0.0003 | 0.0005 | 0.0005 |
| | 0.0004 | 0.0003 | 0.0003 | 0.0002 | 0.0003 | 0.0004 |
| STD. DEVIATIONS | 0.0001 | 0.0000 | 0.0000 | 0.0006 | 0.0010 | 0.0009 |
| | 0.0006 | 0.0007 | 0.0007 | 0.0005 | 0.0006 | 0.0006 |

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 4

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| AVERAGES | 0.0001 | 0.0000 | 0.0003 | 0.0036 | 0.0018 | 0.0007 |
| | 0.0002 | 0.0000 | 0.0001 | 0.0006 | 0.0022 | 0.0023 |
| STD. DEVIATIONS | 0.0003 | 0.0000 | 0.0009 | 0.0015 | 0.0013 | 0.0007 |
| | 0.0003 | 0.0002 | 0.0003 | 0.0013 | 0.0021 | 0.0019 |

DAILY AVERAGE HEAD ON TOP OF LAYER 6

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| AVERAGES | 0.0000 | 0.0000 | 0.0000 | 0.0003 | 0.0002 | 0.0001 |
| | 0.0000 | 0.0000 | 0.0000 | 0.0001 | 0.0002 | 0.0002 |
| STD. DEVIATIONS | 0.0000 | 0.0000 | 0.0001 | 0.0001 | 0.0001 | 0.0001 |
| | 0.0000 | 0.0000 | 0.0000 | 0.0001 | 0.0002 | 0.0002 |

DAILY AVERAGE HEAD ON TOP OF LAYER 8

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| AVERAGES | 0.0000 | 0.0000 | 0.0001 | 0.0009 | 0.0008 | 0.0004 |
| | 0.0001 | 0.0000 | 0.0000 | 0.0002 | 0.0007 | 0.0007 |
| STD. DEVIATIONS | 0.0001 | 0.0000 | 0.0002 | 0.0004 | 0.0003 | 0.0002 |
| | 0.0001 | 0.0001 | 0.0002 | 0.0004 | 0.0006 | 0.0005 |

DAILY AVERAGE HEAD ON TOP OF LAYER 10

| | | | | | | |
|-----------------|--------|--------|--------|--------|--------|--------|
| AVERAGES | 0.0001 | 0.0000 | 0.0001 | 0.0021 | 0.0019 | 0.0011 |
| | 0.0002 | 0.0001 | 0.0001 | 0.0005 | 0.0016 | 0.0018 |
| STD. DEVIATIONS | 0.0003 | 0.0000 | 0.0004 | 0.0010 | 0.0007 | 0.0005 |
| | 0.0004 | 0.0002 | 0.0004 | 0.0010 | 0.0014 | 0.0012 |

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| AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100 | | | | |
|---|---------|------------|-----------|----------|
| | INCHES | | CU. FEET | PERCENT |
| PRECIPITATION | 39.20 | (4.823) | 142279.7 | 100.00 |
| RUNOFF | 8.779 | (2.3674) | 31868.48 | 22.398 |
| EVAPOTRANSPIRATION | 24.302 | (2.5984) | 88217.59 | 62.003 |
| LATERAL DRAINAGE COLLECTED FROM LAYER 3 | 6.00731 | (2.22963) | 21806.545 | 15.32654 |
| PERCOLATION/LEAKAGE THROUGH LAYER 4 | 0.11115 | (0.03784) | 403.480 | 0.28358 |
| AVERAGE HEAD ON TOP OF LAYER 4 | 0.001 | (0.000) | | |
| PERCOLATION/LEAKAGE THROUGH LAYER 6 | 0.11115 | (0.03784) | 403.480 | 0.28358 |
| AVERAGE HEAD ON TOP OF LAYER 6 | 0.000 | (0.000) | | |
| PERCOLATION/LEAKAGE THROUGH LAYER 8 | 0.11115 | (0.03784) | 403.480 | 0.28358 |
| AVERAGE HEAD ON TOP OF LAYER 8 | 0.000 | (0.000) | | |
| LATERAL DRAINAGE COLLECTED FROM LAYER 9 | 0.11115 | (0.03780) | 403.465 | 0.28357 |
| PERCOLATION/LEAKAGE THROUGH LAYER 11 | 0.00000 | (0.00000) | 0.015 | 0.00001 |
| AVERAGE HEAD ON TOP OF LAYER 10 | 0.001 | (0.000) | | |
| PERCOLATION/LEAKAGE THROUGH LAYER 12 | 0.00307 | (0.00595) | 11.137 | 0.00783 |
| CHANGE IN WATER STORAGE | -0.008 | (1.3026) | -27.54 | -0.019 |

| PEAK DAILY VALUES FOR YEARS 1 THROUGH 100 | | |
|---|----------|------------|
| | (INCHES) | (CU. FT.) |
| PRECIPITATION | 4.40 | 15972.000 |
| RUNOFF | 4.194 | 15225.1016 |

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| | | |
|--|----------|------------|
| DRAINAGE COLLECTED FROM LAYER 3 | 0.34412 | 1249.16895 |
| PERCOLATION/LEAKAGE THROUGH LAYER 4 | 0.004427 | 16.06958 |
| AVERAGE HEAD ON TOP OF LAYER 4 | 0.020 | |
| MAXIMUM HEAD ON TOP OF LAYER 4 | 0.030 | |
| LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN) | 0.0 FEET | |
| PERCOLATION/LEAKAGE THROUGH LAYER 6 | 0.004427 | 16.06958 |
| AVERAGE HEAD ON TOP OF LAYER 6 | 0.002 | |
| PERCOLATION/LEAKAGE THROUGH LAYER 8 | 0.004427 | 16.06958 |
| AVERAGE HEAD ON TOP OF LAYER 8 | 0.005 | |
| DRAINAGE COLLECTED FROM LAYER 9 | 0.00361 | 13.08806 |
| PERCOLATION/LEAKAGE THROUGH LAYER 11 | 0.000000 | 0.00022 |
| AVERAGE HEAD ON TOP OF LAYER 10 | 0.010 | |
| MAXIMUM HEAD ON TOP OF LAYER 10 | 0.019 | |
| LOCATION OF MAXIMUM HEAD IN LAYER 9 (DISTANCE FROM DRAIN) | 0.0 FEET | |
| PERCOLATION/LEAKAGE THROUGH LAYER 12 | 0.000289 | 1.04834 |
| SNOW WATER | 9.62 | 34911.7891 |
| MAXIMUM VEG. SOIL WATER (VOL/VOL) | 0.4355 | |
| MINIMUM VEG. SOIL WATER (VOL/VOL) | 0.2000 | |

*** Maximum heads are computed using McEnroe's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner
by Bruce M. McEnroe, University of Kansas
ASCE Journal of Environmental Engineering
Vol. 119, No. 2, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100

| LAYER | (INCHES) | (VOL/VOL) |
|-------|----------|-----------|
| 1 | 2.7516 | 0.4586 |
| 2 | 9.2275 | 0.3845 |
| 3 | 0.0020 | 0.0100 |

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| | | |
|------------|----------|--------|
| 4 | 0.0000 | 0.0000 |
| 5 | 0.1920 | 0.0320 |
| 6 | 0.0510 | 0.8500 |
| 7 | 0.0184 | 0.3070 |
| 8 | 150.8400 | 0.4190 |
| 9 | 0.3840 | 0.0320 |
| 10 | 0.0000 | 0.0000 |
| 11 | 2.1900 | 0.3650 |
| 12 | 1.5163 | 0.2527 |
| SNOW WATER | 0.046 | |

